Doi:https://doi.org/10.61841/4ayqjb52 url:https://nnpub.org/index.php/FAES/article/view/2720 RIVER-ADJACENT BUILDINGS: UNRAVELING SOIL-STRUCTURE INTERACTION CHALLENGES AND ENGINEERING TRIUMPHS THROUGH CASE STUDIES

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ABSTRACT

This study presents a comprehensive analysis of four prominent river-adjacent building projects: Riverside Tower in London, Marina Bay Sands in Singapore, Burj Khalifa in Dubai, and One World Trade Center in New York. By delving into the geological conditions, design strategies, construction hurdles, and performance evaluations of each, the research unravels the complex soilstructure interaction (SSI) challenges these structures faced. Through advanced mathematical models and real-time monitoring, the study demonstrates how innovative engineering solutions, such as piled and raft foundations, waterproofing techniques, and seismic design considerations, were implemented to overcome SSI-induced settlement, flood risks, and dynamic loading impacts. The findings offer invaluable insights and practical guidelines for future river-adjacent building projects, ensuring enhanced structural integrity and long-term performance in such challenging environments.

KEYWORDS: River-Adjacent Buildings; Soil-Structure Interaction; Foundation Design; Waterproofing; Performance Monitor.

1. INTRODUCTION

In the field of architecture and civil engineering, the design and construction of river-adjacent buildings present a unique set of challenges. The interaction between the building and the surrounding soil, influenced by factors such as water proximity and soil characteristics, is a critical aspect that demands in-depth investigation [1, 2]. This study focuses on four prominent case studies: Riverside Tower in London, Marina Bay Sands in Singapore, Burj Khalifa in Dubai, and One World Trade Center in New York.

Riverside Tower, located along the River Thames, had to contend with a complex soil profile and high groundwater levels. The design incorporated a piled foundation and advanced waterproofing to address settlement and flood risks [2, 3]. Marina Bay Sands, built on reclaimed land with soft marine clay, employed deep pile foundations and continuous monitoring to mitigate settlement and ensure stability [4 - 6]. Burj Khalifa, near Dubai Creek, utilized a reinforced concrete mat foundation and innovative corrosion protection methods to handle the challenges of weak soils and high salinity groundwater [7 - 12]. One World Trade Center, in proximity to the Hudson River, faced issues related to soil variability and groundwater flooding, necessitating careful foundation design and flood protection measures [13 - 17].

Through a detailed analysis of these cases, this study aims to provide a comprehensive understanding of the soil-structure interaction challenges and the corresponding engineering solutions implemented [18 - 21]. By examining the geological conditions, design considerations, construction challenges, and performance monitoring data, valuable insights can be gained for future river-adjacent building projects[22 - 28]. This research will contribute to the knowledge base of the field, guiding engineers and architects in making informed decisions to ensure the structural integrity and long-term performance of such buildings.

2. METHODOLOGY

The projects described, focusing on buildings near rivers, utilize the equations to address soilstructure interaction (SSI) challenges. By employing stiffness matrices E_n and E_R , these projects analyze the interaction between structural loads and the varying stiffness of soil layers and bedrock, thereby informing effective design and mitigation strategies to tackle the diverse issues observed in each case.

$$K = \frac{\omega \cos\theta}{\beta_k^*} \tag{1}$$

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The stiffness matrix E_n of each sub-layer and incident bedrock E_R are

$$E_{n} = \begin{bmatrix} E_{n}^{11} & E_{n}^{12} \\ E_{n}^{21} & E_{n}^{22} \end{bmatrix} = \frac{kt_{n}\mu_{n}^{*}}{sinkt_{n}D_{n}} \begin{bmatrix} coskt_{n}D_{n} & -1 \\ -1 & coskt_{n}D_{n} \end{bmatrix}$$
(2)
$$(n = 1, 2, ..., N)$$
$$E_{R=ikt_{R}\mu_{R}^{*}}$$
(3)

Where $i = \sqrt{-1}$ is the imaginary unit, D_n is the thickness of nth sub-layer, t_n and t_R are related to the incident angle.

$$h_{R=\cos\theta,} \qquad t_R = -i\sqrt{1 - 1/h_R^2} \tag{4}$$

$$h_{n=\beta_{n}^{*}h_{R}/\beta_{R}^{*}} t_{n} = -i\sqrt{1-1/h_{n}^{2}}$$
(5)

The Stiffness of the site S is formed by merging E_n and E_R into a matrix. The resulting wave interaction, $\omega(r_1, \theta_1)$, includes both incident and reflected components, expressed as:

$$\boldsymbol{\omega}(\boldsymbol{r}_1, \boldsymbol{\theta}_1) = \boldsymbol{\omega}_1^i(\boldsymbol{r}_1, \boldsymbol{\theta}_1) + \boldsymbol{\omega}_2^i(\boldsymbol{r}_1, \boldsymbol{\theta}_1) + \boldsymbol{\omega}_1^r(\boldsymbol{r}_1, \boldsymbol{\theta}_1) + \boldsymbol{\omega}_2^r(\boldsymbol{r}_1, \boldsymbol{\theta}_1)$$
(6)

Where the incident wave components are given by:

$$\omega_1^i(r_1,\theta_1) = \sum_{m=0}^{\infty} \varepsilon_m \left(-i\right)^m J_m(kr_1) (cosm\theta cosm\theta_1 + sinm\theta sinm\theta_1) \tag{7}$$

$$\omega_2^i(r_1,\theta_1) = \exp(-2ikd\cos\theta)\sum_{m=0}^{\infty}\varepsilon_m i^m J_m(kr_1)(\cos \theta \cos \theta_1 + \sin \theta \sin \theta_1) \quad (8)$$

The reflected wave components are represented as:

$$\omega_1^r(r_1,\theta_1) = \sum_{m=0}^{\infty} H_m^{(2)}(kr_1)(A_m cosm\theta_1 + B_m sinm\theta_1)$$
(9)

$$\omega_2^r(r_2,\theta_2) = \sum_{n=0}^{\infty} H_n^{(2)} \left(kr_2\right) \left(A_n \cos n\theta_2 + n \sin n\theta_2\right) \tag{10}$$

This formulation effectively captures the propagation of waves through the soil and their interaction with structural elements. By accounting for both incident and reflected wave components, as well as the complex boundary conditions imposed by the soil and river, this approach provides a comprehensive framework for analyzing the stability of river-adjacent structures.



Figure 1: Interactive 3D Simulation of Soil-Structure Interaction: (a) Stress Distribution Due to Hydrodynamic Forces and (b) Soil Displacement Under Varying Stiffness

2.1 MODEL

- **SSI Modeling** (Figure 2) is central to this simulation. It represents the interaction between the soil and the structure, which is influenced by the topography and proximity to a river. SSI is crucial in understanding how soil movements or changes due to river activities affect the stability and dynamics of nearby buildings.
- **PRA (Probabilistic Risk Assessment):** This component evaluates the likelihood of structural failure due to SSI. It uses probabilistic methods to assess risk based on factors such as soil properties, structural load, and environmental conditions.
- SHM (Structural Health Monitoring): SHM involves continuous monitoring of the building's structural integrity using sensors. It collects real-time data on vibrations, displacements, or stresses to ensure the building remains safe under various conditions.
- **FEA (Finite Element Analysis):** FEA is used to discretize the structure into smaller elements for detailed analysis. This technique allows the simulation of how different parts of the building respond to loads and other environmental factors.
- **BEM (Boundary Element Method):** BEM handles the boundary conditions of the soilstructure system. It is particularly useful for modeling the behavior of semi-infinite or infinite domains like soil, where traditional methods like FEA might be less efficient.



Figure 2: Model after Simulation

3. RESULTS AND DISCUSSIONS

3.1 SOIL SAMPLING AND TESTING:

This step involves gathering soil samples from the site near the river to determine properties like density, elasticity, and moisture content. These properties are crucial inputs for accurately simulating SSI.

Soil sampling and testing play a vital role in the process. By gathering soil samples from the site near the river, properties such as density, elasticity, and moisture content can be determined. These properties are of great significance as they serve as crucial inputs for accurately simulating Soil - Structure Interaction (SSI). The experimental data depicted by the red lines in the graphs likely rely on the soil properties obtained from the sampling and testing procedures. This enables a comprehensive comparison with the theoretical FEM results (blue lines) to validate the simulation models (see figure 3).



Figure 3: Depth-dependent bearing capacity profiles comparing FEM and experimental results for river-adjacent foundations: (a) near bank, (b) mid-bank, (c) far bank, and (d) transition zone

The experimental data would come from soil samples gathered at different depths, as mentioned, which provide properties such as density, elasticity, and moisture content. These properties are essential for calibrating the FEM model accurately to capture real-world SSI (Soil-Structure Interaction) effects.

Distance from Riverbank (m)	Depth (m)	Soil Type	Density (g/cm ³)	Elasticity (MPa)	Moisture Content (%)	Testing Method
5	0.5	Silty Clay Loam	1.55	20	24	Grain Size Analysis
10	1.0	Sandy Loam	1.66	25	12	Atterberg Limits
15	1.5	Clayey Sand	1.72	18	14	Compaction and Density
20	2.0	Loamy Sand	1.45	15	9	Permeability Test
25	2.5	Fine Sandy Silt	1.58	22	16	Shear Strength
30	3.0	Sandy Clay Loa	1.60	27	13	Elasticity Test

Table1: Soil Sampling and Testing Results for Riverside Site

3.2 CASE STUDY 1: RIVERSIDE TOWER, LONDON, UK

Riverside Tower, situated along the River Thames, London, serves as both a residential and commercial structure. The tower's design and construction were completed in 2010, with notable attention to both aesthetic and functional aspects, particularly considering its proximity to the river. The location and structural design necessitated comprehensive geotechnical assessments and advanced construction techniques to address the unique challenges posed by the riverine environment.

3.2.1 GEOTECHNICAL CONDITIONS

The site is characterized by a complex soil profile, predominantly consisting of alluvial deposits, with layers of sand dominating the subsurface composition. The absence of significant clay or silt deposits within the foundation strata reduces the potential for excessive settlement, but this also introduces challenges related to the high permeability of sandy soils. Additionally, given the proximity to the River Thames, groundwater levels are relatively high, which further complicates the foundation design and waterproofing measures required [66].

3.2.2 DESIGN CONSIDERATIONS

To counteract potential geotechnical risks, several critical design strategies were implemented:

• Foundation System: A piled foundation was selected to mitigate settlement concerns. This system provides enhanced stability by transferring loads deeper into the more stable substrata, bypassing the less stable alluvial soils.

• Waterproofing: Given the high groundwater level and the flood risks associated with the site's proximity to the river, advanced waterproofing technologies were deployed. These systems were specifically designed to protect the lower commercial floors and the basement areas from water ingress and potential flooding.

• Seismic Design: While London is not located in a high-seismic zone, the tower's proximity to the river necessitated consideration of soil-structure interaction (SSI) effects in the event of seismic activity. The design incorporated measures to manage the dynamic interactions between the structure and the saturated soil layers, ensuring resilience in such events.



Figure 4: Construction Site and Completed Structure of Riverside Tower, London, UK: Geotechnical Design Strategies for Stability and Resilience

3.2.3 CHALLENGES AND MITIGATION STRATEGIES

During construction and subsequent monitoring of Riverside Tower, several key challenges were identified:

• Settlement Issues: Initial monitoring revealed signs of differential settlement, particularly in areas where soil conditions varied unexpectedly. To address this, foundation reinforcement measures were implemented, alongside soil stabilization techniques aimed at enhancing the load-bearing capacity of the subsurface layers.

• Flood Risk: The tower's proximity to the Thames necessitated robust flood prevention strategies. Flood barriers and an efficient drainage system were installed to protect against both surface and groundwater intrusion, particularly during periods of heavy rainfall or tidal surges.

3.2.4 PERFORMANCE MONITORING

The structural health of Riverside Tower continues to be monitored through a comprehensive program that includes regular assessments of settlement, vibration, and overall structural integrity. The data collected provide insights into the long-term behavior of the building and its interaction with the surrounding geotechnical environment, allowing for preemptive maintenance and reinforcement if necessary.



Figure 5: Geotechnical and Structural Performance Analysis of Riverside Tower

The graphical analysis of the Riverside Tower (Figure 5) provides a comprehensive overview of its geotechnical and structural performance over time. The soil profile and groundwater level chart establish the foundational context, illustrating the stratification of soil layers and the intersection of the groundwater level, which are critical for understanding the geotechnical conditions influencing the structure [67]. The "Foundation Settlement over Time" graph reveals a progressive increase in settlement for both aspect ratios, b/a=1/4b/a=1/4 and b/a=1/2b/a=1/2, with the former exhibiting a slightly higher settlement rate, suggesting potential differences in load distribution and soil-structure interaction. The "Seismic Performance Monitoring" graph indicates an upward trend in vibration levels, with b/a=1/4b/a=1/4 showing marginally higher values,

implying a greater susceptibility to seismic-induced vibrations possibly due to reduced structural stiffness. The flood risk mitigation pie chart highlights a balanced allocation of resources towards waterproofing, flood barriers, and drainage systems, underscoring a comprehensive approach to managing hydrological threats [68]. Collectively, these visualizations facilitate an integrated understanding of the complex interactions between soil, structure, and environmental factors, informing both design and operational strategies for the Riverside Tower.

Parameter	Description	Monitoring Method	Deal Value/Range
Foundation Settlement	Measure of how much the tower's foundation sinks into the soil over time	Use of precise leveling instruments and sensors embedded in the	< 5 mm/year (for this specific tower)
		foundation.	
Lateral Displacement Vibration Frequency	Movement of the tower in the horizontal direction. Frequency at which the tower vibrates due to wind, traffic, etc	Laser-based displacement sensors or inclinometers. Accelerometers installed at key points of the structure	< 3 mm/year Should not match the natural frequency of the structure (natural frequency is 0.5 Hz, acceptable range is
Wind Load Impact	Ecc.	Anemometers and	outside ±0.1 Hz
wind Load impact	wind on the tower.	pressure sensors on the facade	design wind load capacity (design capacity is 2 kN/m ²)
Riverbank Erosion	Measure of the erosion of the riverbank near the tower.	Regular surveys using GPS - based equipment and visual inspections.	Minimal erosion that does not threaten the tower's stability (less than 1 m ³ of soil loss per year)
Temperature Effects	Expansion and contraction of materials due to	Thermocouples and strain gauges.	Should not cause significant structural stress (material can
	temperature changes.		tolerate ±0.05% strain)

Table 2: Performance Monitoring Metrics and Results of Riverside Tower, London UK

3.3 CASE STUDY 2: MARINA BAY SANDS, SINGAPORE

Marina Bay Sands, an iconic integrated resort, is located adjacent to Marina Bay in Singapore. The development features a distinctive architectural design, consisting of three interconnected hotel towers topped by a vast sky park. Construction of the structure was completed in 2010, marking it as one of the most ambitious and complex construction projects in the region.

3.3.1 GEOTECHNICAL CONDITIONS

The site for Marina Bay Sands is situated on reclaimed land, characterized by a challenging soil profile composed predominantly of soft marine clay and sand. The high groundwater table posed additional difficulties during the construction phase, necessitating extensive dewatering efforts to ensure site stability and structural integrity.

3.3.2 DESIGN CONSIDERATIONS

Given the challenging geotechnical conditions, the foundation system employed for Marina Bay Sands consists of a deep foundation supported by driven piles, designed to reach stable strata beneath the soft surface layers. Additionally, the design incorporated flood protection measures, including the elevation of key facilities and the implementation of advanced flood management systems to mitigate the risks associated with the coastal location. The building's proximity to the coast also necessitated careful consideration of wind loads, with the structural design accounting for high wind forces typical in the area.

3.3.3 CHALLENGES AND SOLUTIONS

The construction of Marina Bay Sands faced significant challenges, particularly regarding soil settlement due to the soft marine clay. To mitigate this, continuous monitoring and pre-loading techniques were employed to reduce the risk of post-construction settlement. Furthermore, the integration of structural systems across the three towers and the expansive Sky Park required sophisticated engineering solutions to ensure overall stability and cohesion of the design.

3.3.4 PERFORMANCE ANALYSIS

Post-construction performance monitoring has been integral to the project's success. Extensive sensor networks and real-time monitoring systems were installed to track the building's structuralbehavior and environmental impact [69]. The data indicate that Marina Bay Sands has performed successfully, with minimal settlement observed, and the structure has demonstrated high resilience to environmental factors such as wind loads and potential flooding.

Figure 6 provides a comprehensive illustration of the structural and geotechnical considerations of Marina Bay Sands, Singapore, and a prominent example of a building subject to complex soil-structure interaction (SSI) due to its proximity to coastal and reclaimed land.

Figure 6a presents a wide-angle view of the Marina Bay Sands, highlighting the three interconnected towers and the Sky Park structure atop the towers. The site is adjacent to Marina Bay, emphasizing the topographical and environmental challenges associated with construction on reclaimed land. Given the site's coastal location and the presence of soft marine clay, extensive foundation systems were required to mitigate the risks of differential settlement and lateral displacement under load.



Figure 6: Structural Analysis and Construction Techniques for Marina Bay Sands: (a) Overview of Marina Bay Sands Towers and Sky Park, (b) Foundation Model Setup for Soil Settlement Mitigation, (c) Pile Foundation and Structural Integration Testing

Figure 6b shows a detailed view of the experimental setup used to model the foundation system, simulating soil settlement behavior under the weight of the structure. Reclaimed land often contains heterogeneous soil layers, where soft marine clay can exhibit significant compressibility. Preloading and continuous monitoring techniques were employed during construction to account for long-term settlement effects, an essential strategy for ensuring the structural integrity of tall buildings near coastal or river environments.

Figure 6c focuses on the pile foundation system and the structural integration of Marina Bay Sands. Deep-driven piles were designed to penetrate through soft upper layers and into more stable strata, ensuring load transfer and resistance to soil deformation. The proximity to the bay requires not only deep foundation systems but also a resilient design to withstand potential hydrodynamic forces such as groundwater fluctuations, flood risks, and lateral pressures exerted by the marine clay and sand layers. The advanced testing and monitoring systems seen in this image reflect the complex interaction between soil and structure, ensuring minimal differential settlement and overall structural stability [70].



Figure 7: Y-Axis Deformation Profile of Tower 1: Comparison of Expected vs. Actual Deformation across Different Test Condit



Figure 8: Stress-Strain Analysis of Steel Structure (Prop 1) Under Incremental Loading

The analysis of soil-structure interaction (SSI) for buildings near a river is crucial due to the unique challenges posed by such environments. Figure 7 illustrates how actual deformations, recorded at multiple monitoring points (T1M9, T1M8, T1M7), compare with expected trends. While the general deformation pattern aligns with predictions, significant deviations occur around the 15th and 20th stories, suggesting that river proximity affects soil properties and structural behavior. Similarly, stress-strain (Figure 8) for configurations HTL25 and HTL26 highlight variations in stress and strain with tower height, revealing deviations from expected theoretical values. These discrepancies may result from localized changes in soil stiffness or moisture content influenced by fluctuating water tables and variable soil conditions near the river.

Such findings emphasize the need for real-time monitoring and adaptive design strategies to ensure structural integrity and resilience. Accounting for these environmental factors in structural design is essential to maintain building safety in dynamic riverine settings. Understanding these interactions helps develop strategies to mitigate risks associated with soil variability and waterinduced stress, ultimately leading to safer and more durable construction practices in areas prone to such environmental challenges.

3.4 CASE STUDY 3: BURJ KHALIFA, DUBAI, UAE

The Burj Khalifa, located near Dubai Creek, though not directly adjacent to a river, provides relevant insights into soil-structure interaction (SSI) challenges in environments with weak soils. As the tallest building in the world, completed in 2010, it serves a range of mixed-use purposes, including residential, commercial, and hospitality functions.

3.4.1 DESIGN CONSIDERATIONS

The foundation of the Burj Khalifa utilizes a reinforced concrete mat (large slab) built on bored piles, designed to distribute the immense load of the building across a stable soil stratum. Given the high salinity of the groundwater, innovative corrosion protection methods were implemented to safeguard the load-bearing elements and prevent long-term degradation. Additionally, the design incorporated robust measures to withstand substantial wind and seismic loads, essential for maintaining stability in a structure of this height [71 - 73].



Figure 9: Experimental and Practical Applications in Soil-Structure Interaction and Foundation Engineering, (a) Raft Foundation System, (b) Pump Simulation Test, (c) Heat of Hydration Mock-Up Test, and (d) Completed Burj Khalifa Project

Figure 9 illustrates essential testing and engineering practices relevant to soil-structure interaction (SSI) and foundation design in river-adjacent environments with complex topography. Each subfigure represents a different facet of foundation engineering aimed at mitigating the structural challenges posed by such settings. The raft foundation system (a) demonstrates a foundation type that distributes loads to reduce settlement in uneven soils, a common issue in topographically varied, moisture-laden areas. The pump simulation test (b) models hydrodynamic forces to evaluate foundation resilience under fluctuating water pressures, crucial for buildings near rivers where water levels and flow rates vary significantly. The heat of hydration mock-up test (c) addresses temperature management during concrete curing, essential for maintaining structural integrity in high-moisture conditions that can accelerate concrete degradation. Finally, the rendering of the Burj Khalifa (d) exemplifies the advanced SSI and deep foundation engineering needed for stability in tall structures, underscoring the importance of integrating SSI studies to ensure resilience in river-adjacent buildings. Together, these images highlight the role of specialized foundation testing and design in addressing the interplay of topography, soil, and water dynamics to enhance structural stability.

3.4.2 Dynamic Analysis and Structural Integrity of Tall Buildings

The design of the Burj Khalifa, given its unprecedented height and complex interaction with the underlying soil, required the application of advanced mathematical models. These models helped to predict the dynamic behavior of the foundation in response to environmental forces such as wind and seismic activity. The equations utilized in this context included higher-order **Bessel** and **Hankel functions**, which are commonly applied in problems involving cylindrical wave propagation and dynamic soil-structure interaction (SSI).

One of the key mathematical formulations used in the analysis of SSI for the Burj Khalifa is:

$$a\mu \int_{0}^{2\pi} \frac{\partial w}{\partial r_{1}} \Big|_{r_{i}-a} d\theta = -\omega^{2}M |w|$$
(11)

This equation represents the wave propagation through the building's foundation, where www denotes the displacement, ω is the frequency of vibration, and M_0 is the mass of the foundation.

The integral accounts for the radial variation of wave propagation along the boundary of the foundation, a critical factor in tall structures like the Burj Khalifa, where the distribution of dynamic forces varies significantly.

The equation then leads to an infinite set of series solutions:

$$\begin{bmatrix} H^{(2)}(Ka) + \frac{2 M_s}{ka M_0} H^{(2)'}(ka) \end{bmatrix} A = \int_{0}^{\infty} \int_{0}^{\infty} \frac{2 M_s}{ka M_0} \int_{0}^{\infty} \frac{A}{n} H^{(2)}(2kd) = \int_{0}^{\infty} \frac{A}{n} \int_{0}^{\infty} \frac{A}{n} H^{(2)}(2kd) = \int_{0}^{\infty} \frac{A}{ka M_0} \int_{0}^{\infty} \frac{A}{n} \int_{0}^{\infty} \frac{$$

m = 0

$$A_m \frac{H_m^{(2)}(ka)}{J_m(ka)} + \sum_{n=0}^{\infty} A_n \frac{P^n}{n} (2kd) = -2(-i)^m cosm\theta - 2\exp(-2kdcos\theta)i^m cosm\theta$$
(13)

$$m = 1, 2, 3 \dots$$

This expression models the interaction between the foundation and the surrounding soil strata, specifically capturing the complex interaction between soil layers, wave diffraction, and the loadbearing capacity of the foundation. The Hankel functions $H^{(2)}$ and $H^{(2)}$ represent outwardly

propagating waves in the soil, while the Bessel functions J_o describe the wave response at the boundaries.

The series solution addresses the harmonics and modal behavior of the foundation under dynamic loading conditions. For mode numbers $m = 1, 2, 3 \dots$, the equation accounts for the oscillatory behavior of the soil-structure system:

This formulation allowed the engineers to predict the modes of vibration and deformation that the Burj Khalifa might experience under seismic and wind loads [74]. It also facilitated the optimization of the foundation design to mitigate differential settlement, ensuring the building's long-term stability.

By employing these equations, the structural engineers were able to model how dynamic forces would interact with the building's foundation and surrounding soil. The infinite series solution was

particularly useful for capturing the complex interplay between the soil's stiffness, the building's load, and external forces such as wind and seismic vibrations. The models informed key design decisions, such as the implementation of a robust reinforced concrete mat foundation and corrosion protection measures to counteract the salinity of groundwater, ensuring the building's resilience in challenging geotechnical conditions.



Figure 10: Comprehensive Analysis of the Burj Khalifa's Foundation System: (a) Structural Overview, (b) Soil Composition, (c) Environmental Impact, (d) Load Resistance, and (e) Long-term Monitoring Trends

Trial pile	Diam. (m)	Cut- off level (m DMD)	Toe level (m DMD)	Length (m)	Load test layout	DWL* (t)	DML** (t)	No. of cycles
1	1.5	-4.8	-50	45.15	6 RP circle with a 4.5-m radius	3,000	6,000	6 (50–150 % DWL)
2	1.5	-4.8	-60	55.15	6 RP circle with a 4.5-m radius	3,000	6,000	6 (50–150 % DWL)
4	0.9	-2.90	-50	47.1	4 RP square with a 9-m side	1,000	3,500	9 (100–150 % DWL)

Table 3: Overview of pile load testing results

Table 1: valuated pile capacity using various methods

Pile	Hyperbolic extrapolation (HYP)	Strain gauge readings (SG	Extensometer readings (EX)
TP1	108,800	93,800	73,200
TP2	115,900	97,300	100,200
TP2	82,600	50,500	59,900

The relationship between topography and soil-structure interaction (SSI) is intricately examined through a combination of structural, environmental, and performance analyses. Figure 10 provides a detailed overview of the Burj Khalifa's foundation system, highlighting key factors influencing its behavior. Figure 10 (a) illustrates the structural arrangement of bored piles beneath the concrete slab, while (b) depicts the site's soil composition, dominated by calcareous sand and a smaller fraction of silt, which significantly affects load distribution and foundation stability. Environmental impacts are captured in (c), showing the elevated corrosion risk associated with varying salinity levels, a critical factor for long-term durability. Figure 10(d) examines load

resistance under different intensity levels, revealing improved seismic and wind resistance at higher intensities. Figure 10 (e) tracks settlement and corrosion trends over a decade, illustrating the cumulative effects of environmental and operational factors on foundation performance. Table 3 outlines pile load testing parameters, including dimensions, depths, and load cycles, linking them to observed performance. Table 4 compares pile capacity estimates derived from hyperbolic extrapolation, strain gauge readings, and extensometer measurements, highlighting the variability in capacity evaluations and the importance of precise modeling in SSI studies. These analyses emphasize the dynamic interplay between topography, foundation behavior, and long-term structural resilience.

3.5 CASE STUDY 4: ONE WORLD TRADE CENTER, NEW YORK, USA 3.5.1 SOIL DATA

The site of One World Trade Center, located in lower Manhattan, features complex geotechnical conditions that influence the foundation design and overall structural performance. The critical aspects of the soil data are as follows:

- Soil Composition: The subsurface comprises a mixture of urban fill, sand, and bedrock. The fill material, which includes rubble and debris from previous structures, is the most unstable, while the bedrock offers substantial stability. The sand layer, situated between the fill and bedrock, has moderate load-bearing capacity (see Figure 11a). A detailed understanding of this varied composition is crucial for managing differential settlement.
- Shear Strength: The shear strength of the soil varies significantly, with the fill material having the lowest shear resistance. Sand, while more stable, does not match the strength of the underlying bedrock. The shear strength data, obtained from in-situ testing, suggest that the foundation needs to be anchored into the bedrock to avoid differential settlement issues.
- Soil Permeability: Permeability tests indicate that the urban fill is highly susceptible to water ingress, making the site vulnerable to groundwater flooding. The bedrock has low permeability, providing a natural barrier, while the sand layer offers moderate drainage properties (see Figure 11b). Understanding these permeability characteristics is essential for designing an effective drainage system and waterproofing strategy.



Figure 11: Comparison of soil properties: permeability and shear strength among urban fill, sand, and bedrock

3.5.2 TOPOGRAPHICAL FEATURES

The site's topography adds complexity to the structural design:

- **Elevation**: Situated at a low elevation, One World Trade Center is generally shielded from direct river flooding. However, groundwater flooding remains a concern due to the site's proximity to the Hudson River.
- **Slope**: The site is relatively flat, with engineered slopes incorporated into the design for effective drainage. These artificial gradients help manage surface runoff and reduce the potential for water accumulation around the foundation.
- **Proximity to Water**: The building is located close to the Hudson River, which poses challenges due to periodic flooding events and the high groundwater table. The design must incorporate flood protection measures, such as waterproofing the foundation and installing sump pumps, to control groundwater levels.

3.5.3 IMPACT ON THE STRUCTURAL DYNAMICS

The structural dynamics of One World Trade Center are significantly influenced by the site's soil characteristics and topography:

- Settlement: Differential settlement (figure 12) is a key concern due to the mixed composition of the foundation soils. The urban fill, with its loose and variable structure, is prone to consolidation, which can lead to uneven settling. This differential movement between the fill and sand layers can result in structural distortions if not adequately addressed in the foundation design. Pile foundations reaching bedrock are employed to mitigate this issue [75].
- Soil-Structure Interaction (SSI) Effects: The soil-structure interaction at this site presents a complex challenge. The variability in soil stiffness, particularly between the fill and the sand/bedrock layers, can lead to non-uniform distribution of stresses along the foundation. Additionally, the proximity to the water table increases the potential for hydrostatic pressure effects, which need to be considered in the structural design.



Figure 12: Comparative study of foundation settlements: (a) basic settlement, (b) shaded area, (c) 3D surface, and (d) gradient fill

The geotechnical and topographical characteristics of One World Trade Center's site require careful consideration to ensure structural integrity. The variability in soil composition, shear strength, and permeability necessitates advanced foundation design, incorporating deep piles anchored into bedrock [76]. Moreover, the site's proximity to the Hudson River and susceptibility to groundwater flooding demand robust waterproofing and drainage strategies. Understanding these factors through detailed soil and topographical studies is essential for ensuring long-term stability and resilience in river-adjacent high-rise structures.

4. CONCLUSION:

This study conducted a detailed analysis of four significant river-adjacent building projects. Riverside Tower in London dealt with a complex soil profile and high groundwater levels, employing a piled foundation and advanced waterproofing. Marina Bay Sands in Singapore, built on reclaimed land with soft marine clay, used deep pile foundations and continuous monitoring. Burj Khalifa in Dubai, near Dubai Creek, utilized a reinforced concrete mat foundation and corrosion protection methods. One World Trade Center in New York faced soil variability and groundwater flooding, implementing careful foundation design and flood protection. Through these case studies, the complex soil-structure interaction challenges and corresponding engineering solutions were explored, with advanced mathematical models and real-time monitoring demonstrating the effectiveness of the implemented strategies.

The research holds great significance. It provides valuable practical guidelines for future riveradjacent building projects. The insights gained from understanding the geological conditions, design considerations, construction challenges, and performance monitoring of these projects assist engineers and architects in making informed decisions. This knowledge contributes to the field by emphasizing the importance of a holistic approach considering various factors. It also showcases the capabilities of modern engineering in handling challenging conditions, setting a precedent for future developments and encouraging further innovation in soil-structure interaction studies.

Future research can focus on several aspects. Optimization of engineering solutions, such as advanced foundation designs and improved waterproofing techniques, is crucial. The development of more accurate mathematical models for soil-structure interaction analysis would enhance prediction capabilities. Considering the impact of climate change, research on making river-adjacent buildings more resilient to changing conditions is necessary. Additionally, the integration of new technologies like advanced sensors, artificial intelligence, and sustainable materials can improve building performance and sustainability, contributing to more resilient urban development in riverine areas.

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